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2                   **Quantification of Lateral Forces in Concrete Crosstie Fastening Systems**

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**ABSTRACT**

Consistent increases in cumulative freight tonnages, combined with the move towards higher-speed intercity passenger rail operation, have placed greater demands on North American railroad infrastructure. Concrete crossties and fastening system components are known to fail at a wide range of life cycle intervals when subjected to demanding loading environments. Such failures can cause track geometry defects, require repetitive maintenance procedures, and present critical engineering challenges. Rail seat deterioration (RSD), the degradation of the concrete material beneath the rail, has been identified through surveys of North American Class I railroads as the most critical engineering challenge of concrete crossties. Shoulder/fastener wear or fatigue was identified through the same survey as the second most critical engineering challenge related to concrete crossties. Lateral forces transferred through the fastening system are thought to be a primary cause of degradation of insulators. The objective of this study is to quantify the demands on the insulator through analysis of the transfer of lateral wheel loads into the fastening system by measuring the magnitude of lateral forces entering the shoulder, a component of the fastening system adjacent to the insulator. The Lateral Load Evaluation Device (LLED) was developed at UIUC to quantify these forces. Data captured by the LLED will assist the rail industry in moving towards the mechanistic design of future fastening systems by quantifying lateral forces in the fastening system under representative loading conditions. Information gained through this study will also lead to a better understanding of the frictional forces at key interfaces in the fastening system.

Preliminary results show that the transfer of lateral wheel loads into the fastening system is highly dependent on the magnitude of lateral wheel loads and the frictional characteristics of the fastening system.

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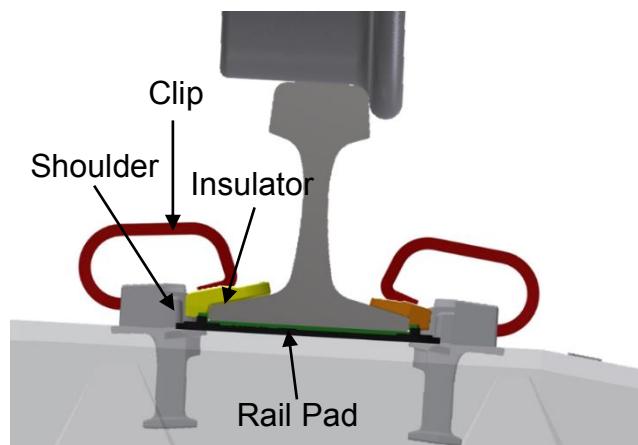
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24 *Keywords:* Lateral force, heavy axle load, friction, concrete crosstie, fastening system design

## 1 INTRODUCTION

2 Concrete crossties and elastic fastening systems are typically installed in demanding loading  
 3 environments, such as lines with heavy axle load (HAL) freight traffic, high degrees of  
 4 curvature, steep grades, extreme climates, higher speed rail traffic, or passenger rail traffic which  
 5 requires strict geometric tolerances. These loading environments may be too demanding for  
 6 conventional timber crossties, limiting their life cycles and increasing the cost effectiveness of  
 7 concrete crossties. Although concrete crossties may provide a better option than conventional  
 8 timber crossties in demanding environments, they are not without their design and performance  
 9 challenges. Rail seat deterioration (RSD), the degradation of the concrete material beneath the  
 10 rail, has been identified through surveys of North American Class I railroads as the most critical  
 11 engineering challenge associated with concrete crossties. Shoulder/fastener wear or fatigue was  
 12 identified through the same survey as the second most critical engineering challenge for concrete  
 13 crossties (1). Shoulder/fastener wear or fatigue causes excessive rail movement, which expedites  
 14 the RSD process.

15 The component located between the rail base and the anchorage point for the elastic clip  
 16 is commonly referred to as an insulator (Figure 1). The insulator is a critical component given it  
 17 contacts nearly every other component within the fastening system. However, though the  
 18 insulator is a critical component, it is also designed to be a sacrificial wear component to prevent  
 19 the rail or shoulder from wearing. As an insulator wears it can lose the ability to maintain track  
 20 gauge, attenuate lateral bearing forces to the shoulder, provide electrical isolation, or transmit the  
 21 design clamping force from the clip to the rail. Furthermore, as an insulator wears, some track  
 22 geometry defects (e.g. wide gauge) will become more prevalent and excessive rail movement can  
 23 occur, accelerating failure mechanisms of other fastening system components or the concrete  
 24 crosstie itself (e.g. rail pad wear and RSD) (2, 3). This presents the engineering challenge of  
 25 designing an insulator that can withstand the demands at this critical interface while also  
 26 maintaining the integrity of the entire fastening system.  
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28  
 29  
 30 **FIGURE 1 Safelok I fastening system component description.**

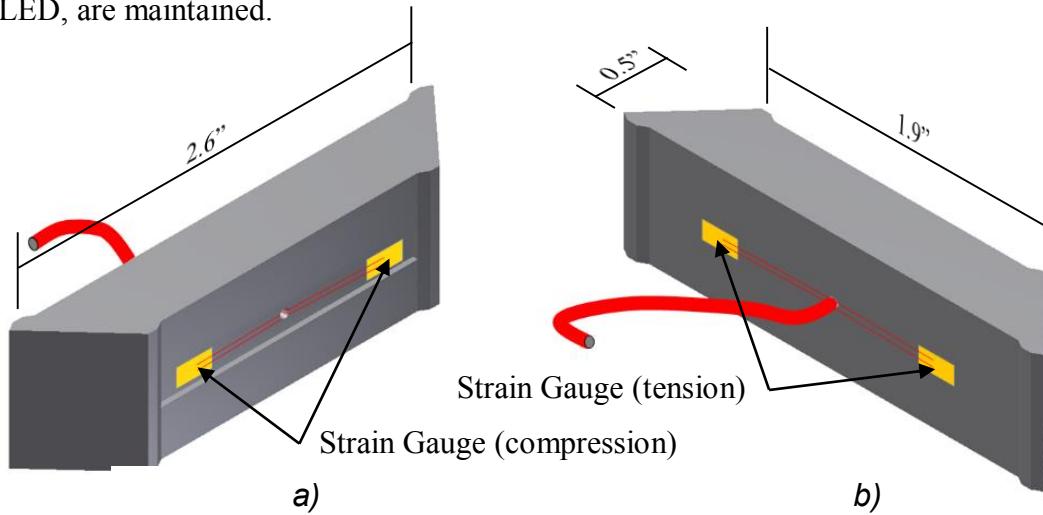
31 A simplified Failure Mode and Effect Analysis (FMEA) was used to guide our approach  
 32 to addressing failed insulators. The FMEA was used to define and identify the modes of failure,  
 33 their causes, and the effects they have on other fastening system components and the system as a  
 34 whole (4). The outcome of the FMEA narrowed our focus to three primary causes of insulator  
 35 failure: abrasion, fracturing, and crushing. Abrasion occurs when relative motion occurs

1 between the insulator and the shoulder or rail base. This relative motion, combined with lateral  
 2 forces acting normal to the insulator, will degrade the insulator. Fracturing of the insulator will  
 3 occur when lateral forces are applied in a way that causes the component to fracture or crack.  
 4 Lastly, crushing occurs when the lateral force acting on the insulator exceeds the material  
 5 property's limits and the insulator deforms plastically. Ultraviolet (UV) light or moisture  
 6 exposure can also alter material properties and initiate failure (5).

7 By quantifying the lateral forces passing through the insulator and bearing on the  
 8 shoulder, we gain valuable insight into the demands placed on it, allowing for mechanistic  
 9 design. Mechanistic design is a process derived from analytical and scientific principles,  
 10 considering field loading conditions and performance requirements (6). University of Illinois at  
 11 Urbana-Champaign (UIUC) researchers have designed the Lateral Load Evaluation Device  
 12 (LLED) to measure the lateral bearing forces acting on the shoulder and aid in the mechanistic  
 13 design of the fastening system and its components.

## 15 MEASUREMENT TECHNOLOGY

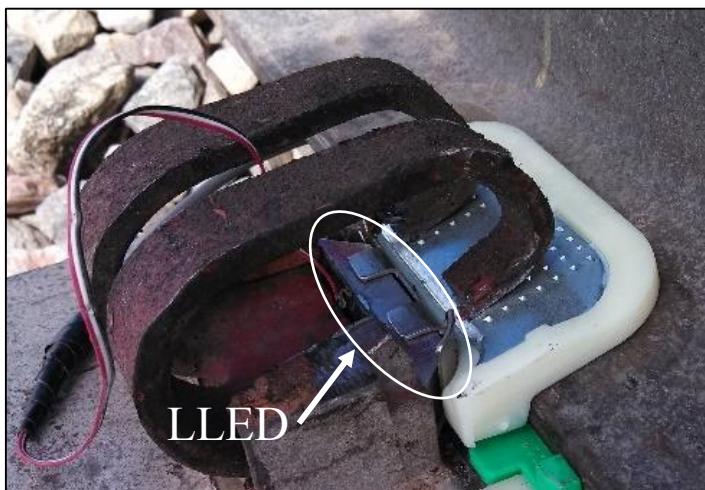
16 UIUC's LLED has two defined points of contact with the shoulder that act as outer supports and  
 17 two defined points of contact with the insulator that are narrower than the supports. Under load,  
 18 this specific geometry induces a bending action of the beam. The beam contains four strain  
 19 gauges which are wired into a full Wheatstone Bridge to measure bending strain under load.  
 20 Two strain gauges are applied horizontally one inch from the center of the beam to measure  
 21 compressive strains (Figure 2a). The locations of the gauges are between the points of contact  
 22 with the insulator to minimize damage to the gauges. The other two strain gauges used to  
 23 measure tensile strains are applied horizontally one inch from the center of the beam between the  
 24 two supports (Figure 2b). The face of the fastening system shoulder is ground away using a  
 25 handheld grinder and straight edge to ensure the original dimensions, after placement of the  
 26 LLED, are maintained.



30 **FIGURE 2 LLED strain gauge location and orientation.**

31  
 32 Once the shoulder face is ground away, the LLED replaces it (Figure 3). The primary  
 33 advantage of this technology is that the original fastening system geometry is maintained, thus  
 34 clip installation procedures and all fastening system components remain the same. Furthermore,

1 material selection and geometry were also designed to reduce experimental error caused by  
2 different stiffnesses than an unaltered fastening system. Because lateral restraint is one of the  
3 fastening system's primary functions, the LLED also will allow researchers to understand how  
4 variables associated with friction (e.g. materials and geometry) alter the lateral load path in  
5 addition to the magnitudes of lateral fastening system forces (7).  
6



7  
8  
9 **FIGURE 3 LLED installed in Safelok I fastening system.**

10  
11 LLED strain values are resolved into a force through calibration curves generated prior to  
12 testing using a uniaxial loading frame. LLEDs were supported on a level plate by two small steel  
13 blocks and loaded with a self-leveling loading head to ensure perpendicular loading during  
14 calibration. Loads were applied in 1,000 pound (1 kilopound (kip)) increments while  
15 corresponding strains were recorded. A thin steel insert is placed between the insulator and the  
16 two points of contact on the beam to ensure the points of loading would not penetrate into the  
17 comparatively soft insulator material (Nylon 6/6). If this did happen, it would turn the two-point  
18 load into a distributed load, negatively impacting the accuracy of the results (6). The stiffness of  
19 the beam and insert were chosen such that the stiffness of the system remained similar to its  
20 original condition. The result is a load cell at the shoulder-insulator interface that preserves the  
21 original geometry and ensures that the load path within the fastening system remains unaffected.  
22

## 23 **BACKGROUND**

24 UIUC and other researchers have succeeded in measuring and quantifying the load path in the  
25 vertical direction through the use of strain gauges and additional instrumentation in the fastening  
26 system (8). Additionally, researchers at UIUC have successfully implemented matrix based  
27 tactile surface sensors (MBTSS) to measure the pressure distribution in the vertical direction at  
28 the interface of the rail pad and concrete rail seat (9). However, there have been few, if any,  
29 attempts to quantify and understand the mechanisms of lateral force restraint in the fastening  
30 system at a level that would guide design and maintenance practices (10). Furthermore, the  
31 mechanisms by which lateral forces are restrained (i.e. bearing or frictional forces) are not well  
32 understood.

33 For the Safelok I fastening system, it is assumed that the majority of the lateral forces  
34 from wheels are restrained by bearing forces (e.g. acting on the shoulder) and frictional forces

1 (e.g. acting between the rail and rail pad and the rail pad and rail seat). This relationship is  
 2 expressed in Equation 1.

3

4  $L_R = \Sigma L_B + \Sigma L_F$  (1)

5

6 where,

7

8  $L_R$  = Total lateral restraining force

9  $\Sigma L_B$  = Summation of lateral bearing forces

10  $\Sigma L_F$  = Summation of lateral frictional forces

11 All lateral bearing forces within a fastening system are measured by the LLED because  
 12 there are no other surfaces for lateral forces to bear on in a Safelok I type fastening system.  
 13 Lateral bearing restraint forces are affected by geometric tolerances within the track structure  
 14 and fastening system as well as the lateral fastening system stiffness (11). Lateral frictional  
 15 restraint forces occur at the interfaces between the rail and rail pad as well as the rail pad and rail  
 16 seat. Lateral frictional restraint forces are affected by the vertical wheel load, fastening system  
 17 component material properties, and their frictional characteristics relative to one another. Lateral  
 18 frictional restraint forces are assumed to be the difference between the applied lateral wheel load  
 19 and the sum of all consecutive LLEDs (i.e. lateral bearing forces). Frictional forces require a  
 20 force normal to the plane of the interface between two surfaces, and the relationship is expressed  
 21 in Equation 2.

22

23  $L_F = \mu N$  (2)

24

25 where,

26

27  $L_F$  = Lateral frictional forces

28  $\mu$  = Coefficient of friction (COF) between rail pad and rail seat

29  $N$  = Vertical wheel load (i.e. force applied normal to frictional planes)

30

31 This paper presents results from field experiments that were designed to understand the  
 32 variables that affect the lateral force restraint mechanisms in the fastening system. To better  
 33 understand the relationship between lateral bearing and frictional restraint forces, the following  
 34 points are investigated:

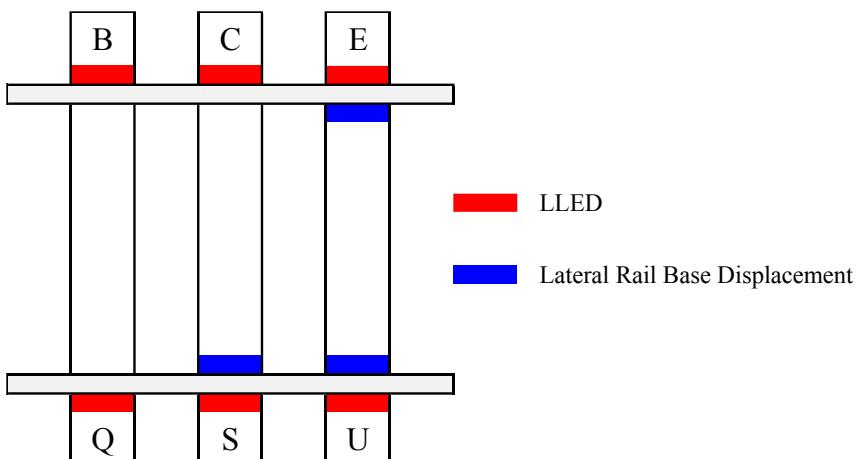
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- 36 ➤ Effect of lateral frictional restraint forces ( $\Sigma L_F$ ) on total lateral restraining force ( $L_R$ ) by  
 37 only varying applied vertical wheel load (i.e. force applied normal to frictional planes)
  - 38 ➤ Percentage of total lateral restraining force ( $L_R$ ) that is restrained by lateral bearing or  
 39 frictional forces ( $\Sigma L_B$  or  $\Sigma L_F$ ) as applied lateral wheel load increases
  - 40 ➤ Lateral restraint forces under dynamic loading
- 41

## 42 EXPERIMENTAL FIELD SETUP

43 Figure 4 shows the location and naming convention of instrumentation within each test section.  
 44 Field experiments and results described in this paper were conducted on a segment of tangent  
 45 track on the Railroad Test Track (RTT) and a segment of curved track on the High Tonnage  
 46 Loop (HTL) at the Transportation Technology Center (TTC) in Pueblo, Colorado. Different

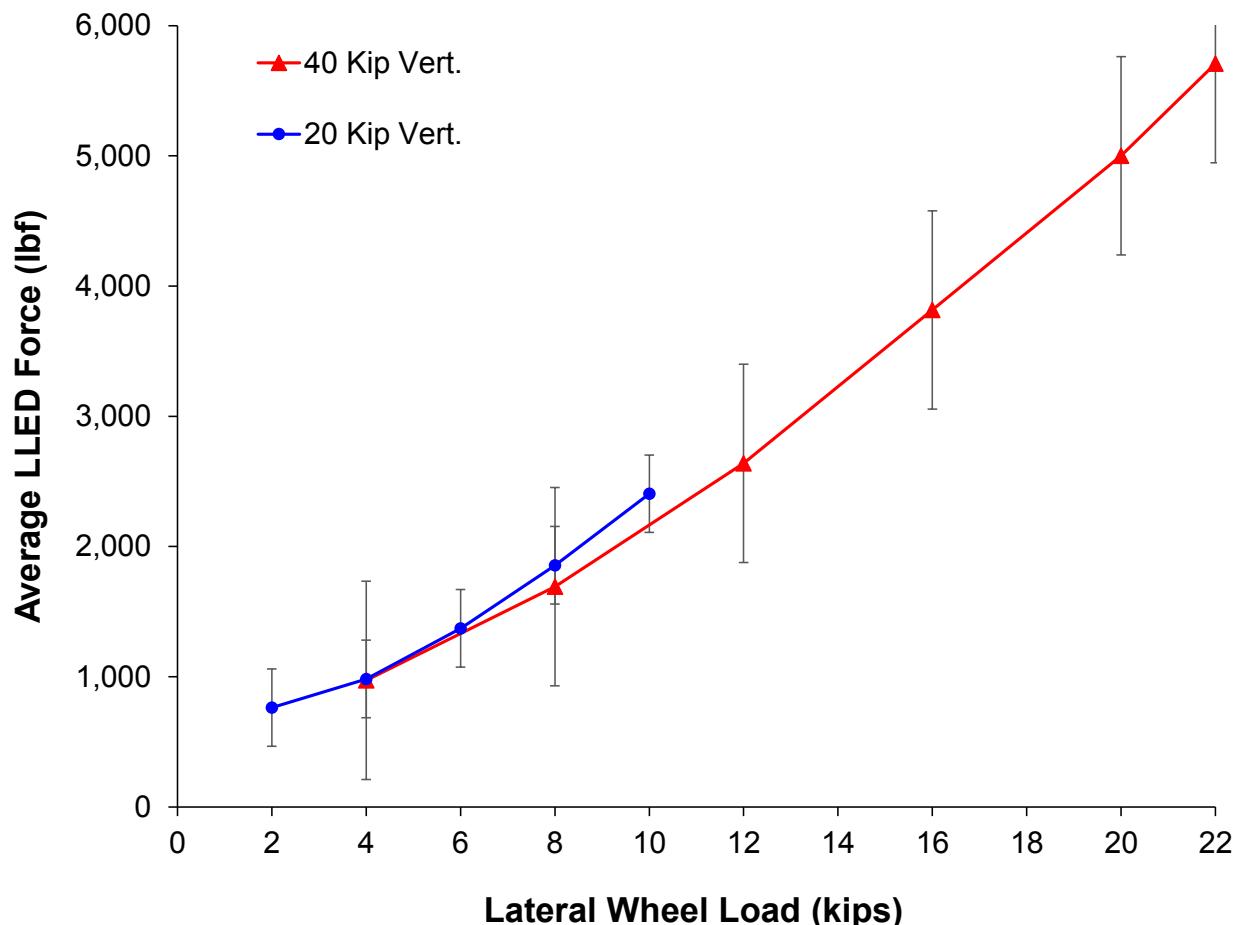
1 static loading scenarios (e.g. load magnitudes, L/V ratios, etc.) were applied to the track using  
 2 the Track Loading Vehicle (TLV) owned by the Association of American Railroads (AAR). The  
 3 TLV uses a deployable axle capable of applying various combinations of vertical and lateral  
 4 loads to simulate typical track loading conditions. A heavy axle load (HAL) freight train was  
 5 used to measure the lateral response of the fastening system under dynamic and impact loading  
 6 conditions on the HTL at speeds of 2, 15, 30, 40, and 45 mph. The HAL freight train consisted  
 7 of three six-axle locomotives and ten freight cars of varying weights and was used to simulate  
 8 the dynamic loading of a freight train. Both test sections consisted of a 136RE rail section,  
 9 concrete crossties spaced at 24 inches center-to-center, Safelok I type fastening systems, and  
 10 premium ballast. LLEDs were installed on the field side of the rail on both rail seats of three  
 11 adjacent concrete crossties. Data were recorded at a sampling rate of 2,000 Hertz to maximize  
 12 the number of samples taken during each static and dynamic test. All instrumentation was  
 13 zeroed after installation and before any experimental data was recorded. Zeroing all  
 14 instrumentation after installation and before experimental runs were conducted removed any  
 15 successive forces or displacements associated with installation procedures, and produced data  
 16 that quantified the applied static forces from the TLV or dynamic forces from passing trains.  
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**FIGURE 4 Instrumentation location and naming convention.**  
 20

21  
**EFFECT OF VARYING APPLIED VERTICAL WHEEL LOAD**  
 22

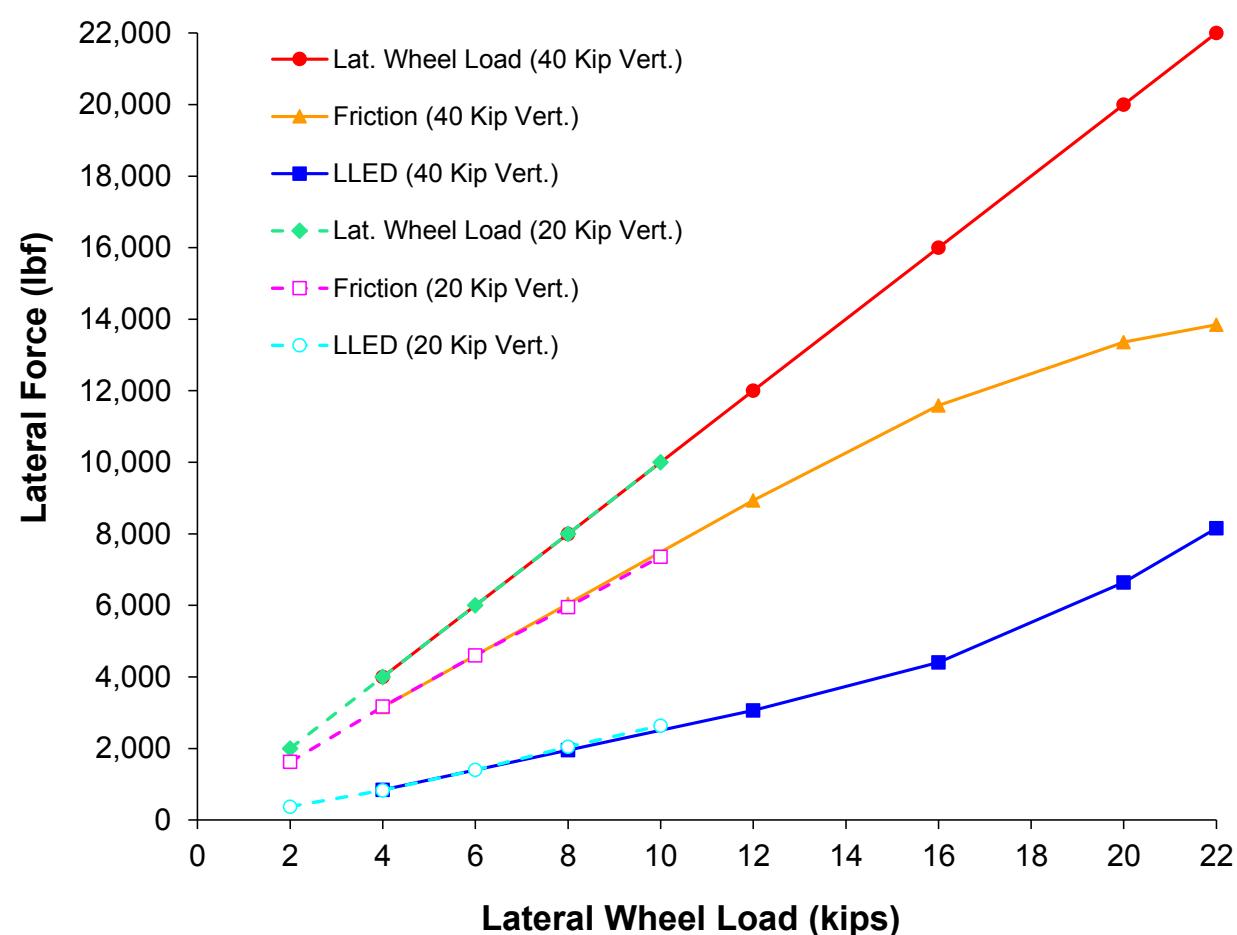
23 The RTT was chosen for static testing to minimize variability due to vehicle-track dynamics in  
 24 the curve. The LLED at rail seat Q on the RTT was compromised during static testing, making  
 25 any data gathered from the LLED unreliable. However, rail seats B, C, E, S, and U functioned  
 26 properly (Figure 4). Data from the five functioning rail seats were analyzed to understand the  
 27 influence of lateral wheel loads on lateral restraint forces in the fastening system. Figure 5  
 28 shows the average magnitude of lateral bearing restraint forces measured by the LLEDs for given  
 29 lateral wheel loads under constant 20 kip and 40 kip vertical wheel loads applied by the TLV  
 30 directly over the specified rail seat. Each data point represents five replicates and the error bars  
 31 represent one standard deviation.  
 32



**FIGURE 5 Average LLED force per rail seat as a function of lateral wheel load.**

The trend of the curves for 20 kip and 40 kip applied vertical wheel loads are similar. As the applied lateral wheel load increases under a constant vertical wheel load, an upward trend of lateral bearing restraint forces (i.e. forces measured by the LLED) can be seen. This is likely due to the theoretically constant frictional force from the constant vertical wheel load under all lateral loading conditions. However, the rate at which the slope of the curve increases appears to be greater under a 20 kip applied vertical wheel load. At four kips of applied lateral wheel load, the LLED force is approximately 1,000 lbf for both vertical loading conditions. As the lateral wheel load increases to eight kips, the LLED forces increase to approximately 1,850 lbf and 1,700 lbf under a 20 kip and 40 kip vertical wheel load, respectively, a difference of 150 lbf. As the lateral wheel load increases to ten kips, the LLED forces increase to approximately 2,400 lbf under a 20 kip vertical wheel load and 2,150 lbf when interpolated under a 40 kip vertical wheel load, a difference of 250 lbf. The increase in the difference between LLED forces under equivalent lateral loads with varied vertical loads indicates that the lower magnitude of vertical wheel load may result in higher lateral bearing restraint forces due to the lower magnitude of force applied normal to the frictional planes. However, this increase in the difference between LLED forces under equivalent lateral loads with varied vertical loads is not in accordance with Equation 2. If the vertical wheel load doubles from 20 kips to 40 kips, the frictional forces should theoretically double, as well, causing the bearing forces at similar lateral loads to decrease.

A reasonable conclusion cannot be drawn from Figure 5 about the effect of vertical wheel loads on both the lateral bearing and frictional restraint forces. This behavior is further confirmed through Figure 6 which shows the sum of lateral forces from rail seats B, C, and E as a function of lateral wheel load under constant 20 kip and 40 kip vertical wheel loads applied by the TLV. Based on Equation 1 and Equation 2, the difference between the lines for total frictional and bearing forces under a 20 kip vertical load should be smaller than under a 40 kip vertical wheel load (i.e. bearing forces should increase and frictional forces should decrease). However, both 20 kip and 40 kip vertical wheel load plots appear to produce similar results for both frictional and bearing force. Because these circumstances do not agree with the theoretical equations, this is an area for future research.



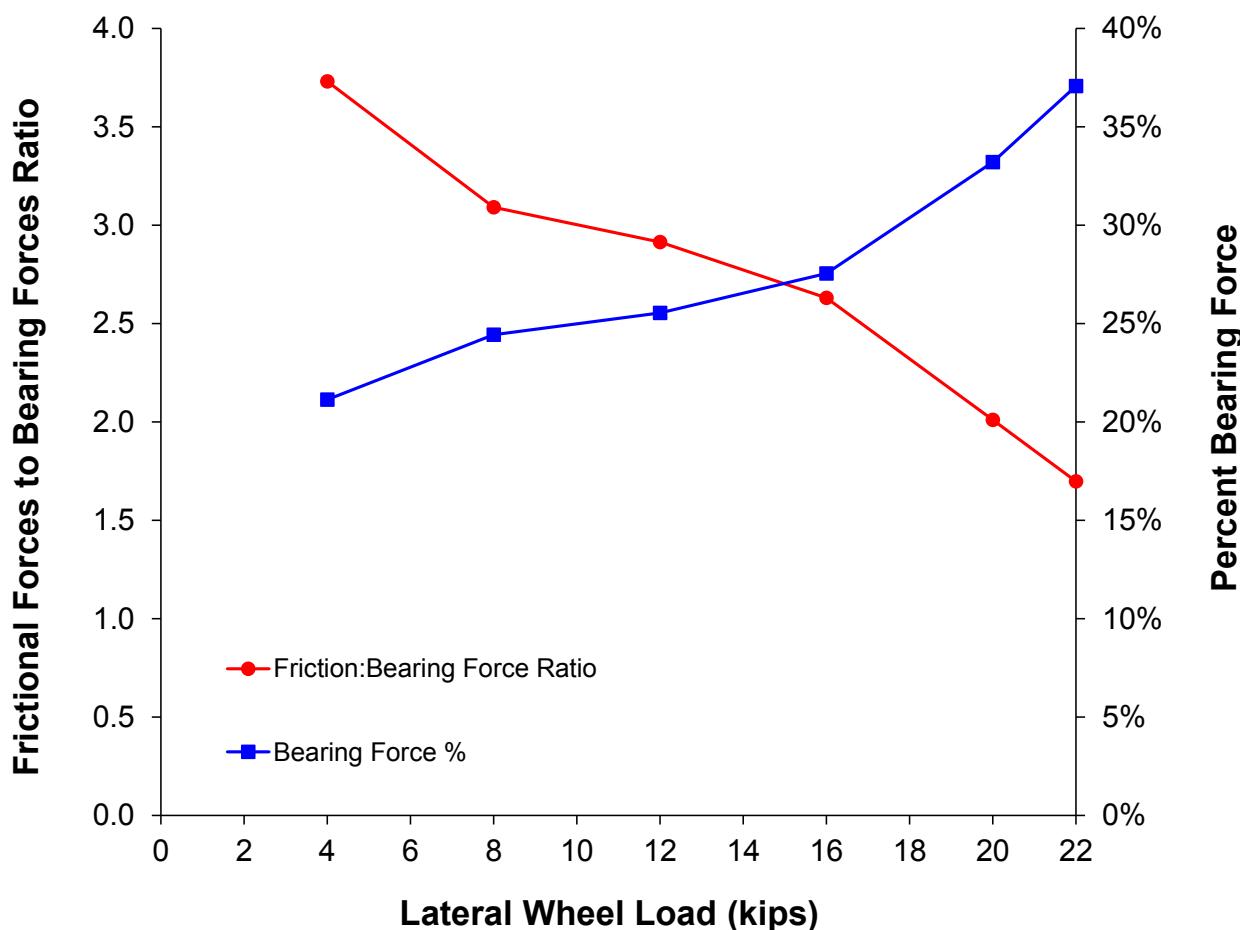
**FIGURE 6 Sum of lateral fastening system forces on rail seats B, C, and E as a function of lateral wheel load.**

#### PERCENTAGE OF LATERAL RESTRAINT FORCES

As previously mentioned, lateral frictional restraint forces are assumed to be the difference between the applied lateral wheel load and the sum of all consecutive LLEDs (i.e. lateral bearing restraint forces). As the applied lateral wheel load increases, the lateral frictional and bearing restraint forces begin to converge (Figure 6). A similar converging trend has also been observed through the analysis of results from UIUC's 3-D finite element (FE) model of the same crosstie and fastening system used in the field (12). The similar results from both field experimentation

1 and FE model data show the converging trend is what occurs within the fastening system as  
 2 lateral wheel load increases. The trend indicates that the percentage of applied lateral wheel load  
 3 restrained by frictional forces decreases while the percentage of applied lateral wheel load  
 4 restrained by bearing forces increases, imparting a higher load on the insulator.

5 Figure 7 shows the change in lateral restraint forces as a function of lateral wheel load in  
 6 two ways: the ratio of frictional forces to bearing forces and lateral bearing restraint forces as a  
 7 percentage. As the applied lateral wheel load increases, the ratio of frictional forces to bearing  
 8 forces decreases from approximately 3.7 at 4 kips of lateral wheel load to 1.7 at 22 kips of lateral  
 9 wheel load, a decrease of 54%. The percentage of the applied lateral wheel load restrained by  
 10 lateral bearing restraint forces increases from approximately 21% at four kips of lateral wheel  
 11 load to 37% at 22 kips of lateral wheel load, an increase of 16%. This indicates that as the lateral  
 12 wheel load increases, the demands on the insulator and shoulder increase due to more of the  
 13 lateral wheel load being restrained by bearing forces.



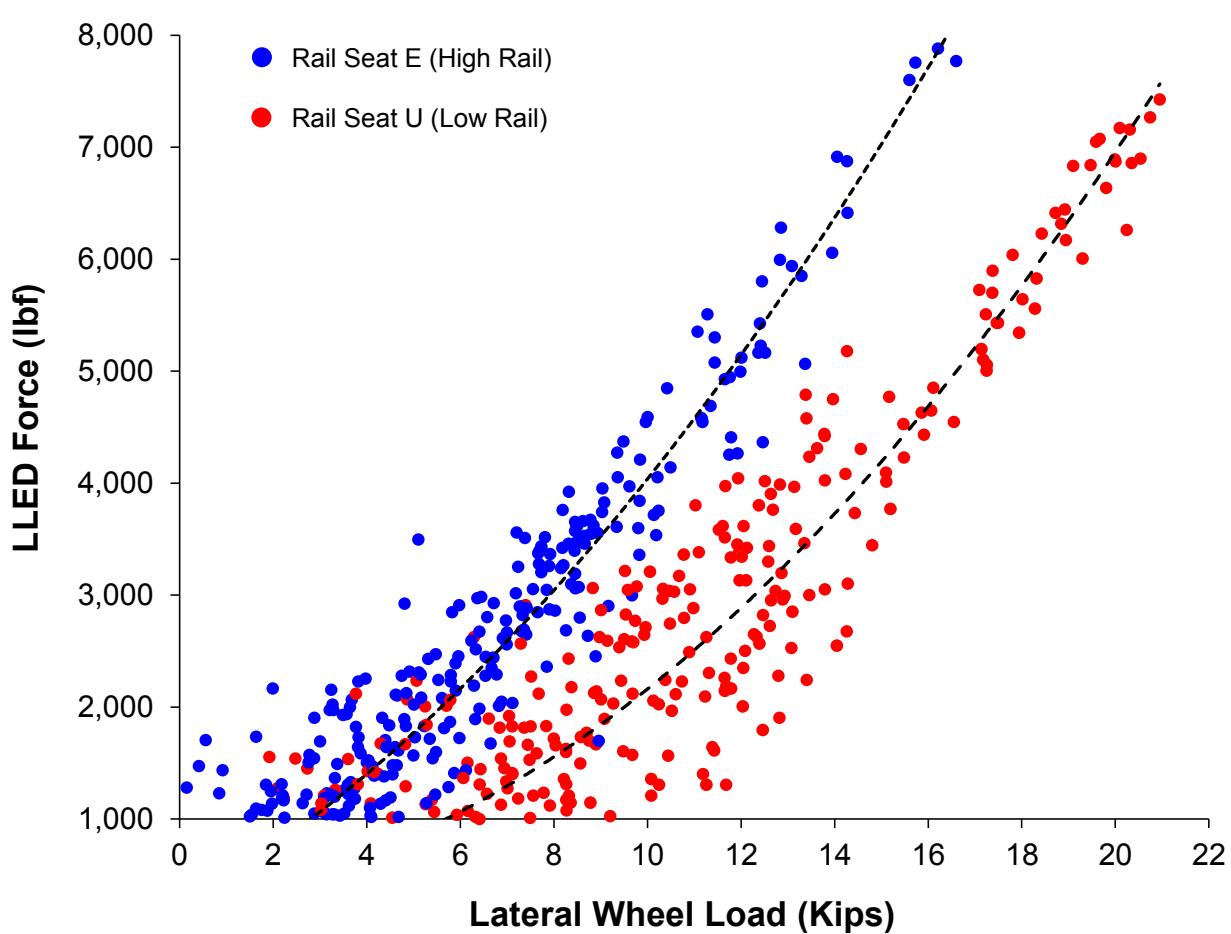
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15 **FIGURE 7 Change in lateral restraint forces as a function of lateral wheel load.**

16 **STATIC VERSUS DYNAMIC LATERAL RESTRAINT FORCES**

17 Figure 8 shows peak LLED forces under a HAL freight train from all axles and tested speeds as a  
 18 function of lateral wheel load. The data indicate that at 20 kips of lateral wheel load, the lateral  
 19 bearing restraint force will be approximately 7,000 lbf for rail seat U and 10,700 lbf when  
 20 extrapolated for rail seat E. This would equate to a percentage of applied lateral wheel load

1 restrained by lateral bearing forces of 35% and 54% for rail seats U and E, respectively.  
 2 Although the data cannot be directly compared due to different testing locations (RTT vs. HTL),  
 3 it can be noted that rail seat U on the low rail of the HTL behaved similarly to the averaged data  
 4 from the RTT. However, rail seat E on the high rail of the HTL produced much higher  
 5 magnitudes of lateral bearing forces than the remaining data. Such a high percentage of lateral  
 6 bearing forces under dynamic loads may be due to the rate of load application from the passing  
 7 train and its effect on the frictional characteristics of the system, as well as the track geometry.  
 8



9  
 10 **FIGURE 8 Peak LLED forces under HAL freight train as a function of lateral wheel load.**

11       Although there appears to be a correlation between lateral bearing forces and lateral  
 12 wheel loads, there is still a large amount of variability between the two metrics. Previous work  
 13 has been conducted to understand the role friction plays on abrasion mechanisms of the concrete  
 14 rail seat, as well as the role lateral fastening system stiffness plays on the magnitudes of lateral  
 15 bearing forces (11, 13). Despite this work, a comprehensive understanding of the role friction  
 16 plays in the transfer of lateral wheel loads into the fastening system does not exist (11, 13). This  
 17 work will be continued with a stronger emphasis on the frictional characteristics of the fastening  
 18 system and its effect on lateral restraint forces in concrete crosstie fastening systems.  
 19  
 20  
 21

## 1 CONCLUSIONS

2 Lateral Load Evaluation Devices (LLEDs) developed at UIUC have proven to be a successful  
3 tool to quantify lateral restraint forces on the shoulder. Two governing equations, Equation 1:  $L_R$   
4  $= \Sigma L_B + \Sigma L_F$ , and Equation 2:  $L_F = \mu N$  guided this research to better understand how lateral  
5 forces are restrained. Static and dynamic observations from the field show a high degree of  
6 correlation and result in the following preliminary conclusions and observations:

- Under equivalent lateral loading conditions, lower magnitudes of vertical wheel load (i.e. force applied normal to frictional planes) may result in higher lateral bearing restraint forces due to the lower magnitude of force applied normal to the frictional planes. However, the difference between constant 20 kip and 40 kip applied vertical wheel load with increasing lateral wheel load is smaller than hypothesized.
- As lateral wheel load increases, the lateral frictional and bearing restraint forces begin to converge. This indicates that the percentage of the applied lateral wheel load restrained by frictional forces ( $\Sigma L_F$  from Equation 1) starts to decrease while the percentage of the applied lateral wheel load restrained by bearing forces ( $\Sigma L_B$  from Equation 1) starts to increase.
- As static lateral wheel load increases, the ratio of frictional forces to bearing forces decreases from approximately 3.7 at 4 kips of lateral wheel load to 1.7 at 22 kips of lateral wheel load, a decrease of 54%. The percentage of the applied lateral wheel load restrained by lateral bearing restraint forces increases from approximately 21% at four kips of lateral wheel load to 37% at 22 kips of lateral wheel load, an increase of 16%.
- In the dynamic lateral loading case, there is increased variability in the lateral loads restrained by bearing forces resulting in increased demands on the fastening system components. For example, the percentage of applied lateral wheel load restrained by lateral bearing forces for rail seats U and E were 35% and 54%, respectively. Though rail seat U exhibits behavior similar to the static results, the bearing force measured from rail seat E exceeds the results from the static analysis by 17%.

In addition to further investigating the role of friction in the performance of the fastening system through controlled laboratory experimentation, future work will include separating the data by rolling stock type (i.e. locomotives, loaded freight cars, empty freight cars, and passenger cars) to investigate the effects of varying train type and their mechanical characteristics. This could provide railroads and suppliers with valuable insight into the demands that mixed traffic impart on the track.

Future fastening system designs will benefit from the presented results by incorporating the data into a mechanistic design approach by designing systems with optimum lateral load transfer characteristics such as material selection and frictional properties. The results from these LLED experiments will also be beneficial in validation of finite element modeling (FEM) work also being conducted at UIUC. Moreover, the results will be used to aid in future fastening system designs to mitigate current recurring problems seen on North American heavy-haul freight railroads.

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