

1 *Manuscript for Annual Meeting Compendium of Papers*

2 **Quantification of Lateral Forces in Concrete Crosstie Fastening Systems**

3 **TRB 15-1534**

4
5 *Transportation Research Board 94th Annual Meeting*

6
7 Submitted: 15 November 2014



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18 3,800 Words, 8 Figures = 5,800 Total Word Count

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1 ABSTRACT

2 Consistent increases in cumulative freight tonnages, combined with the move towards higher-speed
3 intercity passenger rail operation, have placed greater demands on North American railroad infrastructure.
4 Concrete crossties and fastening system components are known to fail at a wide range of life cycle
5 intervals when subjected to demanding loading environments. Such failures can cause track geometry
6 defects, require repetitive maintenance procedures, and present critical engineering challenges. Rail seat
7 deterioration (RSD), the degradation of the concrete material beneath the rail, has been identified through
8 surveys of North American Class I railroads as the most critical engineering challenge of concrete
9 crossties. Shoulder/fastener wear or fatigue was identified through the same survey as the second most
10 critical engineering challenge related to concrete crossties. Lateral forces transferred through the
11 fastening system are thought to be a primary cause of degradation of insulators. The objective of this
12 study is to quantify the demands on the insulator through analysis of the transfer of lateral wheel loads
13 into the fastening system by measuring the magnitude of lateral forces entering the shoulder, a component
14 of the fastening system adjacent to the insulator. The Lateral Load Evaluation Device (LLED) was
15 developed at UIUC to quantify these forces. Data captured by the LLED will assist the rail industry in
16 moving towards the mechanistic design of future fastening systems by quantifying lateral forces in the
17 fastening system under representative loading conditions. Information gained through this study will also
18 lead to a better understanding of the frictional forces at key interfaces in the fastening system.
19 Preliminary results show that the transfer of lateral wheel loads into the fastening system is highly
20 dependent on the magnitude of lateral wheel loads and the frictional characteristics of the fastening
21 system.

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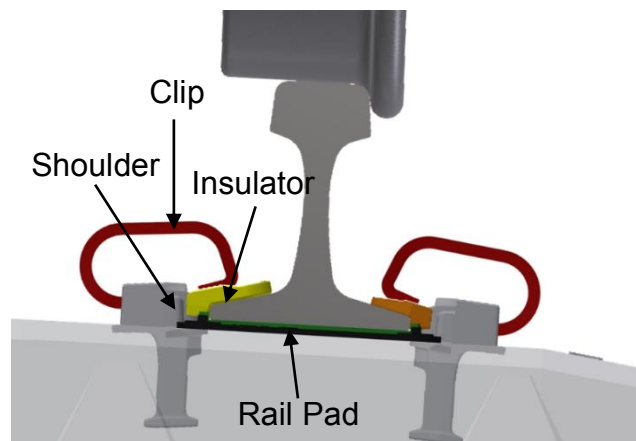
24 *Keywords:* Lateral force, heavy axle load, friction, concrete crosstie, fastening system design

1 INTRODUCTION

2 Concrete cross ties and elastic fastening systems are typically installed in demanding loading
 3 environments, such as lines with heavy axle load (HAL) freight traffic, high degrees of
 4 curvature, steep grades, extreme climates, higher speed rail traffic, or passenger rail traffic which
 5 requires strict geometric tolerances. These loading environments may be too demanding for
 6 conventional timber cross ties, limiting their life cycles and increasing the cost effectiveness of
 7 concrete cross ties. Although concrete cross ties may provide a better option than conventional
 8 timber cross ties in demanding environments, they are not without their design and performance
 9 challenges. Rail seat deterioration (RSD), the degradation of the concrete material beneath the
 10 rail, has been identified through surveys of North American Class I railroads as the most critical
 11 engineering challenge associated with concrete cross ties. Shoulder/fastener wear or fatigue was
 12 identified through the same survey as the second most critical engineering challenge for concrete
 13 cross ties (1). Shoulder/fastener wear or fatigue causes excessive rail movement, which expedites
 14 the RSD process.

15 The component located between the rail base and the anchorage point for the elastic clip
 16 is commonly referred to as an insulator (Figure 1). The insulator is a critical component given it
 17 contacts nearly every other component within the fastening system. However, though the
 18 insulator is a critical component, it is also designed to be a sacrificial wear component to prevent
 19 the rail or shoulder from wearing. As an insulator wears it can lose the ability to maintain track
 20 gauge, attenuate lateral bearing forces to the shoulder, provide electrical isolation, or transmit the
 21 design clamping force from the clip to the rail. Furthermore, as an insulator wears, some track
 22 geometry defects (e.g. wide gauge) will become more prevalent and excessive rail movement can
 23 occur, accelerating failure mechanisms of other fastening system components or the concrete
 24 cross tie itself (e.g. rail pad wear and RSD) (2, 3). This presents the engineering challenge of
 25 designing an insulator that can withstand the demands at this critical interface while also
 26 maintaining the integrity of the entire fastening system.

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30 **FIGURE 1 Safelok I fastening system component description.**

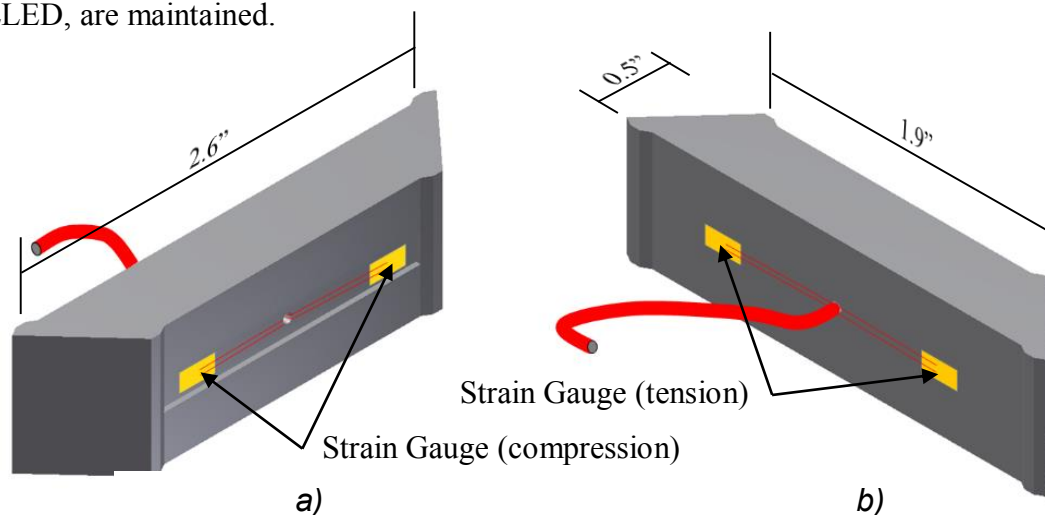
31 A simplified Failure Mode and Effect Analysis (FMEA) was used to guide our approach
 32 to addressing failed insulators. The FMEA was used to define and identify the modes of failure,
 33 their causes, and the effects they have on other fastening system components and the system as a
 34 whole (4). The outcome of the FMEA narrowed our focus to three primary causes of insulator
 35 failure: abrasion, fracturing, and crushing. Abrasion occurs when relative motion occurs

1 between the insulator and the shoulder or rail base. This relative motion, combined with lateral
 2 forces acting normal to the insulator, will degrade the insulator. Fracturing of the insulator will
 3 occur when lateral forces are applied in a way that causes the component to fracture or crack.
 4 Lastly, crushing occurs when the lateral force acting on the insulator exceeds the material
 5 property's limits and the insulator deforms plastically. Ultraviolet (UV) light or moisture
 6 exposure can also alter material properties and initiate failure (5).

7 By quantifying the lateral forces passing through the insulator and bearing on the
 8 shoulder, we gain valuable insight into the demands placed on it, allowing for mechanistic
 9 design. Mechanistic design is a process derived from analytical and scientific principles,
 10 considering field loading conditions and performance requirements (6). University of Illinois at
 11 Urbana-Champaign (UIUC) researchers have designed the Lateral Load Evaluation Device
 12 (LLED) to measure the lateral bearing forces acting on the shoulder and aid in the mechanistic
 13 design of the fastening system and its components.

15 MEASUREMENT TECHNOLOGY

16 UIUC's LLED has two defined points of contact with the shoulder that act as outer supports and
 17 two defined points of contact with the insulator that are narrower than the supports. Under load,
 18 this specific geometry induces a bending action of the beam. The beam contains four strain
 19 gauges which are wired into a full Wheatstone Bridge to measure bending strain under load.
 20 Two strain gauges are applied horizontally one inch from the center of the beam to measure
 21 compressive strains (Figure 2a). The locations of the gauges are between the points of contact
 22 with the insulator to minimize damage to the gauges. The other two strain gauges used to
 23 measure tensile strains are applied horizontally one inch from the center of the beam between the
 24 two supports (Figure 2b). The face of the fastening system shoulder is ground away using a
 25 handheld grinder and straight edge to ensure the original dimensions, after placement of the
 26 LLED, are maintained.



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 29
 30 **FIGURE 2 LLED strain gauge location and orientation.**

31
 32 Once the shoulder face is ground away, the LLED replaces it (Figure 3). The primary
 33 advantage of this technology is that the original fastening system geometry is maintained, thus
 34 clip installation procedures and all fastening system components remain the same. Furthermore,

1 material selection and geometry were also designed to reduce experimental error caused by
2 different stiffnesses than an unaltered fastening system. Because lateral restraint is one of the
3 fastening system's primary functions, the LLED also will allow researchers to understand how
4 variables associated with friction (e.g. materials and geometry) alter the lateral load path in
5 addition to the magnitudes of lateral fastening system forces (7).
6



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9 **FIGURE 3 LLED installed in Safelok I fastening system.**

10
11 LLED strain values are resolved into a force through calibration curves generated prior to
12 testing using a uniaxial loading frame. LLEDs were supported on a level plate by two small steel
13 blocks and loaded with a self-leveling loading head to ensure perpendicular loading during
14 calibration. Loads were applied in 1,000 pound (1 kilopound (kip)) increments while
15 corresponding strains were recorded. A thin steel insert is placed between the insulator and the
16 two points of contact on the beam to ensure the points of loading would not penetrate into the
17 comparatively soft insulator material (Nylon 6/6). If this did happen, it would turn the two-point
18 load into a distributed load, negatively impacting the accuracy of the results (6). The stiffness of
19 the beam and insert were chosen such that the stiffness of the system remained similar to its
20 original condition. The result is a load cell at the shoulder-insulator interface that preserves the
21 original geometry and ensures that the load path within the fastening system remains unaffected.
22

23 **BACKGROUND**

24 UIUC and other researchers have succeeded in measuring and quantifying the load path in the
25 vertical direction through the use of strain gauges and additional instrumentation in the fastening
26 system (8). Additionally, researchers at UIUC have successfully implemented matrix based
27 tactile surface sensors (MBTSS) to measure the pressure distribution in the vertical direction at
28 the interface of the rail pad and concrete rail seat (9). However, there have been few, if any,
29 attempts to quantify and understand the mechanisms of lateral force restraint in the fastening
30 system at a level that would guide design and maintenance practices (10). Furthermore, the
31 mechanisms by which lateral forces are restrained (i.e. bearing or frictional forces) are not well
32 understood.

33 For the Safelok I fastening system, it is assumed that the majority of the lateral forces
34 from wheels are restrained by bearing forces (e.g. acting on the shoulder) and frictional forces

1 (e.g. acting between the rail and rail pad and the rail pad and rail seat). This relationship is
 2 expressed in Equation 1.

$$3 \quad L_R = \Sigma L_B + \Sigma L_F \quad (1)$$

5 where,

7
 8 L_R = Total lateral restraining force
 9 ΣL_B = Summation of lateral bearing forces
 10 ΣL_F = Summation of lateral frictional forces

11 All lateral bearing forces within a fastening system are measured by the LLED because
 12 there are no other surfaces for lateral forces to bear on in a Safelok I type fastening system.
 13 Lateral bearing restraint forces are affected by geometric tolerances within the track structure
 14 and fastening system as well as the lateral fastening system stiffness (*II*). Lateral frictional
 15 restraint forces occur at the interfaces between the rail and rail pad as well as the rail pad and rail
 16 seat. Lateral frictional restraint forces are affected by the vertical wheel load, fastening system
 17 component material properties, and their frictional characteristics relative to one another. Lateral
 18 frictional restraint forces are assumed to be the difference between the applied lateral wheel load
 19 and the sum of all consecutive LLEDs (i.e. lateral bearing forces). Frictional forces require a
 20 force normal to the plane of the interface between two surfaces, and the relationship is expressed
 21 in Equation 2.

$$22 \quad L_F = \mu N \quad (2)$$

24 where,

26
 27 L_F = Lateral frictional forces
 28 μ = Coefficient of friction (COF) between rail pad and rail seat
 29 N = Vertical wheel load (i.e. force applied normal to frictional planes)

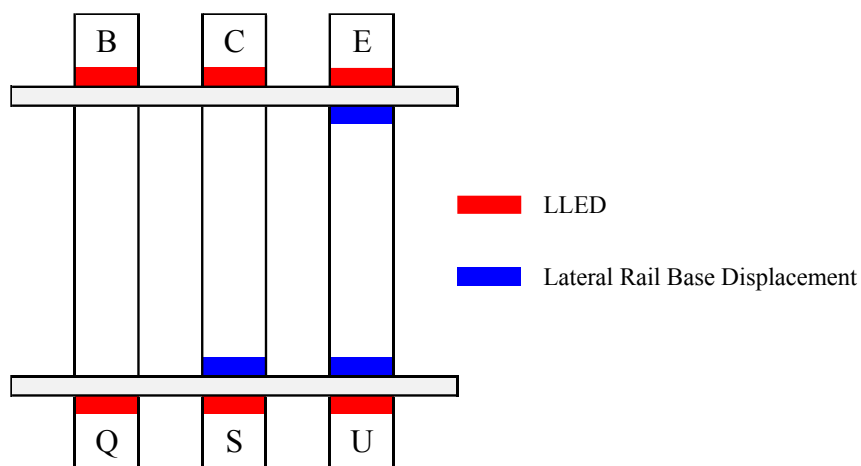
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 31 This paper presents results from field experiments that were designed to understand the
 32 variables that affect the lateral force restraint mechanisms in the fastening system. To better
 33 understand the relationship between lateral bearing and frictional restraint forces, the following
 34 points are investigated:

- 35
 36 ➤ Effect of lateral frictional restraint forces (ΣL_F) on total lateral restraining force (L_R) by
 37 only varying applied vertical wheel load (i.e. force applied normal to frictional planes)
 38 ➤ Percentage of total lateral restraining force (L_R) that is restrained by lateral bearing or
 39 frictional forces (ΣL_B or ΣL_F) as applied lateral wheel load increases
 40 ➤ Lateral restraint forces under dynamic loading

41 42 **EXPERIMENTAL FIELD SETUP**

43 Figure 4 shows the location and naming convention of instrumentation within each test section.
 44 Field experiments and results described in this paper were conducted on a segment of tangent
 45 track on the Railroad Test Track (RTT) and a segment of curved track on the High Tonnage
 46 Loop (HTL) at the Transportation Technology Center (TTC) in Pueblo, Colorado. Different

1 static loading scenarios (e.g. load magnitudes, L/V ratios, etc.) were applied to the track using
 2 the Track Loading Vehicle (TLV) owned by the Association of American Railroads (AAR). The
 3 TLV uses a deployable axle capable of applying various combinations of vertical and lateral
 4 loads to simulate typical track loading conditions. A heavy axle load (HAL) freight train was
 5 used to measure the lateral response of the fastening system under dynamic and impact loading
 6 conditions on the HTL at speeds of 2, 15, 30, 40, and 45 mph. The HAL freight train consisted
 7 of three six-axle locomotives and ten freight cars of varying weights and was used to simulate
 8 the dynamic loading of a freight train. Both test sections consisted of a 136RE rail section,
 9 concrete crossties spaced at 24 inches center-to-center, Safelok I type fastening systems, and
 10 premium ballast. LLEDs were installed on the field side of the rail on both rail seats of three
 11 adjacent concrete crossties. Data were recorded at a sampling rate of 2,000 Hertz to maximize
 12 the number of samples taken during each static and dynamic test. All instrumentation was
 13 zeroed after installation and before any experimental data was recorded. Zeroing all
 14 instrumentation after installation and before experimental runs were conducted removed any
 15 successive forces or displacements associated with installation procedures, and produced data
 16 that quantified the applied static forces from the TLV or dynamic forces from passing trains.
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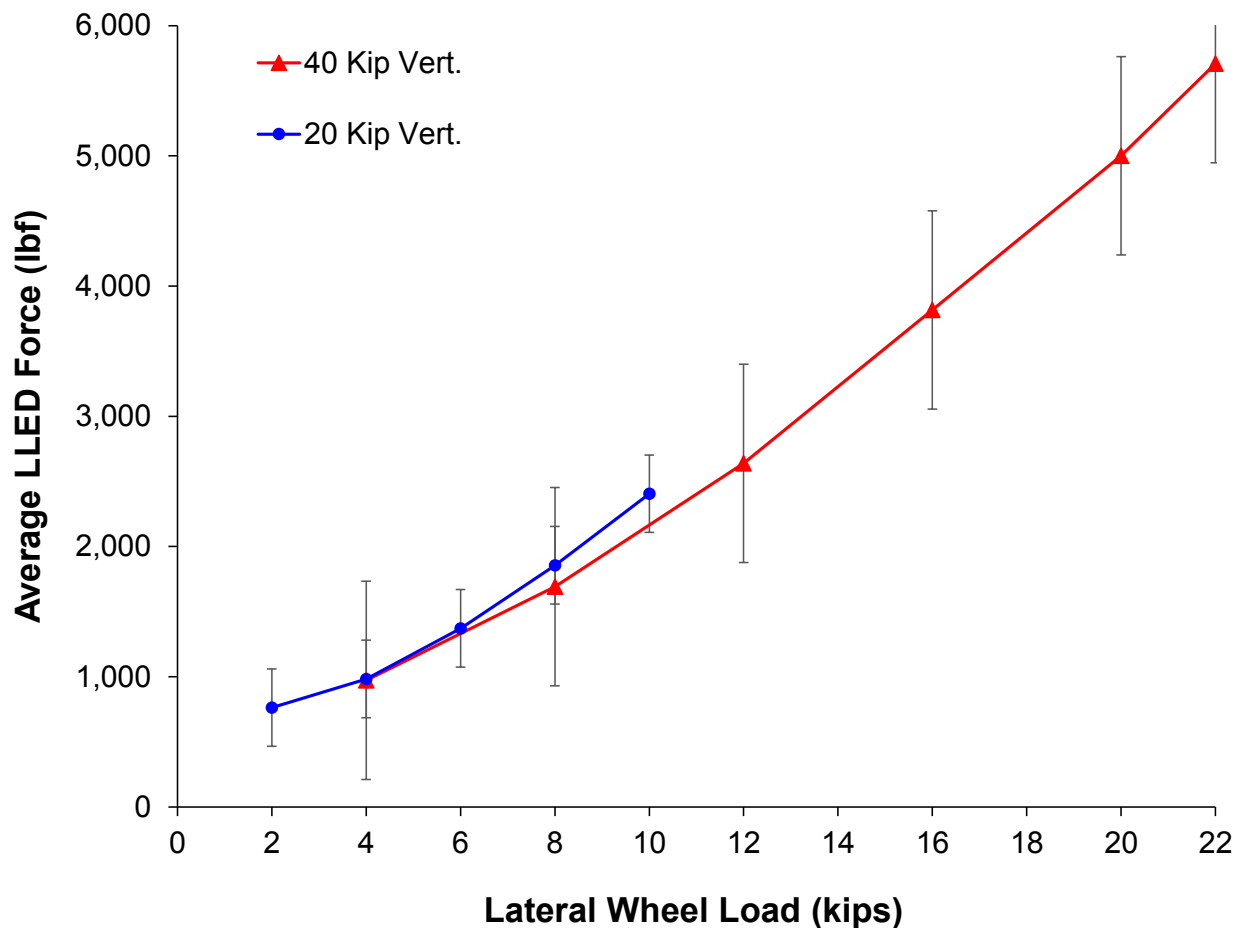
20 **FIGURE 4 Instrumentation location and naming convention.**

21

22 **EFFECT OF VARYING APPLIED VERTICAL WHEEL LOAD**

23

24 The RTT was chosen for static testing to minimize variability due to vehicle-track dynamics in
 25 the curve. The LLED at rail seat Q on the RTT was compromised during static testing, making
 26 any data gathered from the LLED unreliable. However, rail seats B, C, E, S, and U functioned
 27 properly (Figure 4). Data from the five functioning rail seats were analyzed to understand the
 28 influence of lateral wheel loads on lateral restraint forces in the fastening system. Figure 5
 29 shows the average magnitude of lateral bearing restraint forces measured by the LLEDs for given
 30 lateral wheel loads under constant 20 kip and 40 kip vertical wheel loads applied by the TLV
 31 directly over the specified rail seat. Each data point represents five replicates and the error bars
 32 represent one standard deviation.

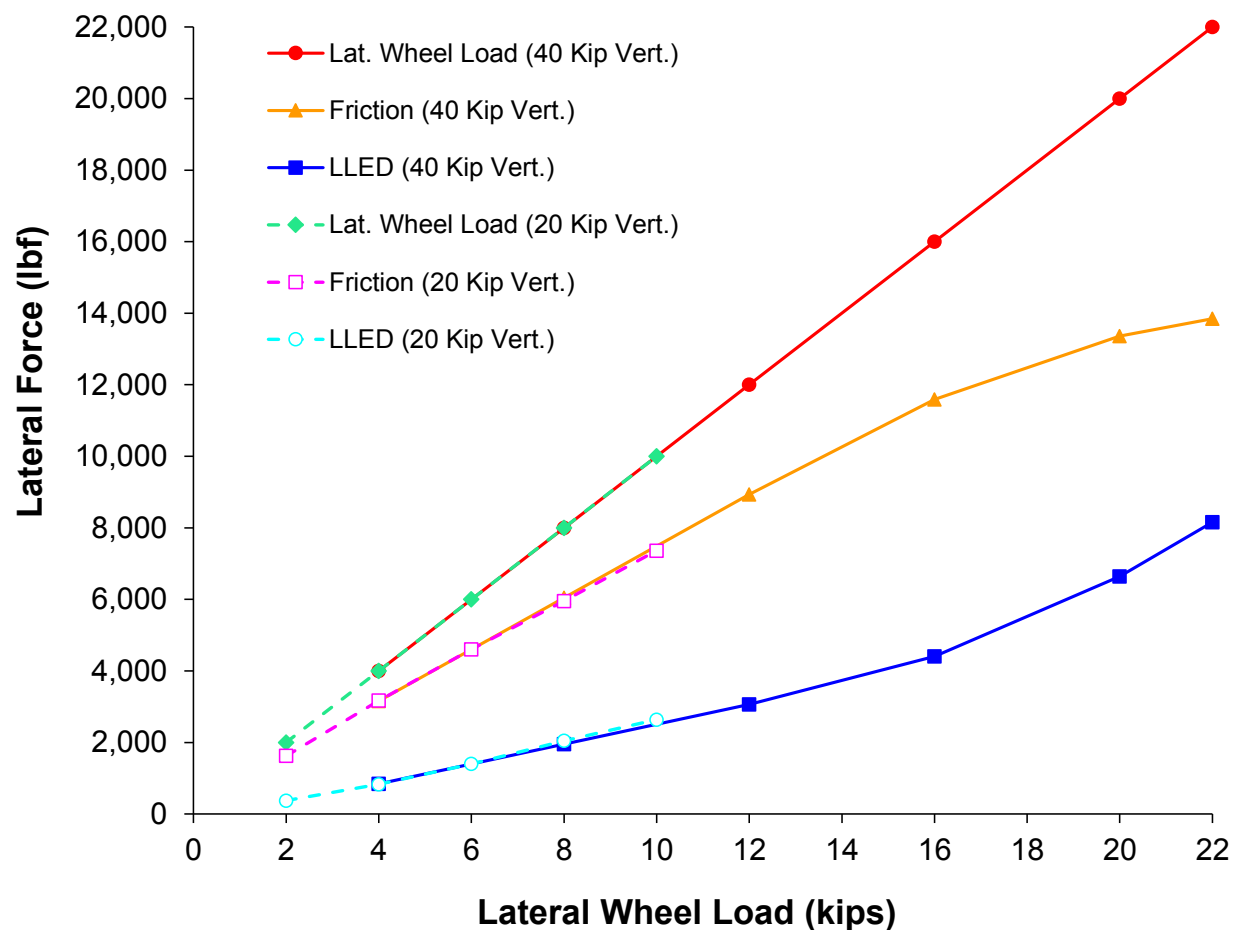


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2 **FIGURE 5 Average LLED force per rail seat as a function of lateral wheel load.**

3 The trend of the curves for 20 kip and 40 kip applied vertical wheel loads are similar. As
 4 the applied lateral wheel load increases under a constant vertical wheel load, an upward trend of
 5 lateral bearing restraint forces (i.e. forces measured by the LLED) can be seen. This is likely due
 6 to the theoretically constant frictional force from the constant vertical wheel load under all lateral
 7 loading conditions. However, the rate at which the slope of the curve increases appears to be
 8 greater under a 20 kip applied vertical wheel load. At four kips of applied lateral wheel load, the
 9 LLED force is approximately 1,000 lbf for both vertical loading conditions. As the lateral wheel
 10 load increases to eight kips, the LLED forces increase to approximately 1,850 lbf and 1,700 lbf
 11 under a 20 kip and 40 kip vertical wheel load, respectively, a difference of 150 lbf. As the lateral
 12 wheel load increases to ten kips, the LLED forces increase to approximately 2,400 lbf under a 20
 13 kip vertical wheel load and 2,150 lbf when interpolated under a 40 kip vertical wheel load, a
 14 difference of 250 lbf. The increase in the difference between LLED forces under equivalent
 15 lateral loads with varied vertical loads indicates that the lower magnitude of vertical wheel load
 16 may result in higher lateral bearing restraint forces due to the lower magnitude of force applied
 17 normal to the frictional planes. However, this increase in the difference between LLED forces
 18 under equivalent lateral loads with varied vertical loads is not in accordance with Equation 2. If
 19 the vertical wheel load doubles from 20 kips to 40 kips, the frictional forces should theoretically
 20 double, as well, causing the bearing forces at similar lateral loads to decrease.

1 A reasonable conclusion cannot be drawn from Figure 5 about the effect of vertical wheel
 2 loads on both the lateral bearing and frictional restraint forces. This behavior is further
 3 confirmed through Figure 6 which shows the sum of lateral forces from rail seats B, C, and E as
 4 a function of lateral wheel load under constant 20 kip and 40 kip vertical wheel loads applied by
 5 the TLV. Based on Equation 1 and Equation 2, the difference between the lines for total
 6 frictional and bearing forces under a 20 kip vertical load should be smaller than under a 40 kip
 7 vertical wheel load (i.e. bearing forces should increase and frictional forces should decrease).
 8 However, both 20 kip and 40 kip vertical wheel load plots appear to produce similar results for
 9 both frictional and bearing force. Because these circumstances do not agree with the theoretical
 10 equations, this is an area for future research.
 11



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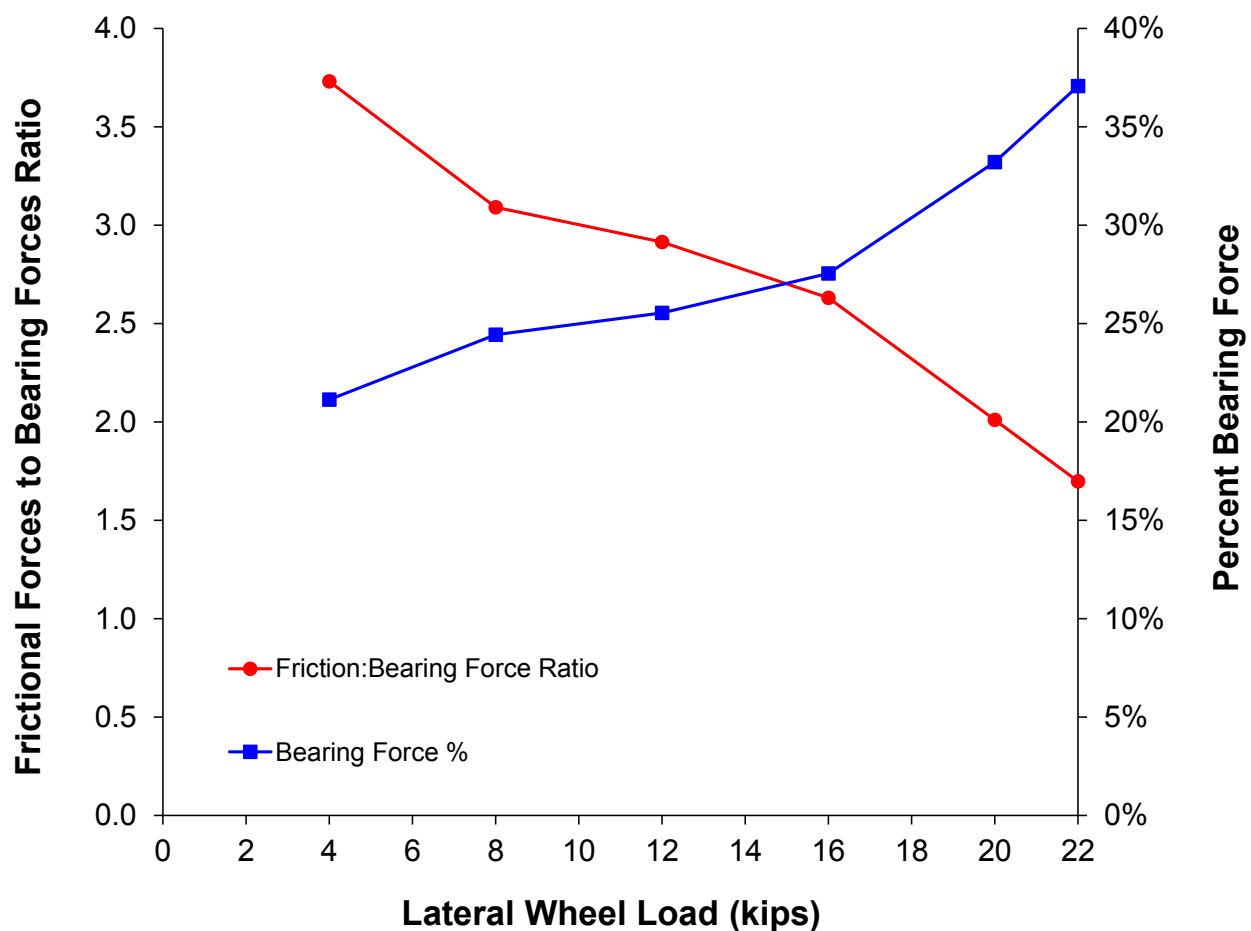
13 **FIGURE 6** Sum of lateral fastening system forces on rail seats B, C, and E as a function of
 14 lateral wheel load.

15 PERCENTAGE OF LATERAL RESTRAINT FORCES

16 As previously mentioned, lateral frictional restraint forces are assumed to be the difference
 17 between the applied lateral wheel load and the sum of all consecutive LLEDs (i.e. lateral bearing
 18 restraint forces). As the applied lateral wheel load increases, the lateral frictional and bearing
 19 restraint forces begin to converge (Figure 6). A similar converging trend has also been observed
 20 through the analysis of results from UIUC's 3-D finite element (FE) model of the same crosstie
 21 and fastening system used in the field (12). The similar results from both field experimentation

1 and FE model data show the converging trend is what occurs within the fastening system as
 2 lateral wheel load increases. The trend indicates that the percentage of applied lateral wheel load
 3 restrained by frictional forces decreases while the percentage of applied lateral wheel load
 4 restrained by bearing forces increases, imparting a higher load on the insulator.

5 Figure 7 shows the change in lateral restraint forces as a function of lateral wheel load in
 6 two ways: the ratio of frictional forces to bearing forces and lateral bearing restraint forces as a
 7 percentage. As the applied lateral wheel load increases, the ratio of frictional forces to bearing
 8 forces decreases from approximately 3.7 at 4 kips of lateral wheel load to 1.7 at 22 kips of lateral
 9 wheel load, a decrease of 54%. The percentage of the applied lateral wheel load restrained by
 10 lateral bearing restraint forces increases from approximately 21% at four kips of lateral wheel
 11 load to 37% at 22 kips of lateral wheel load, an increase of 16%. This indicates that as the lateral
 12 wheel load increases, the demands on the insulator and shoulder increase due to more of the
 13 lateral wheel load being restrained by bearing forces.



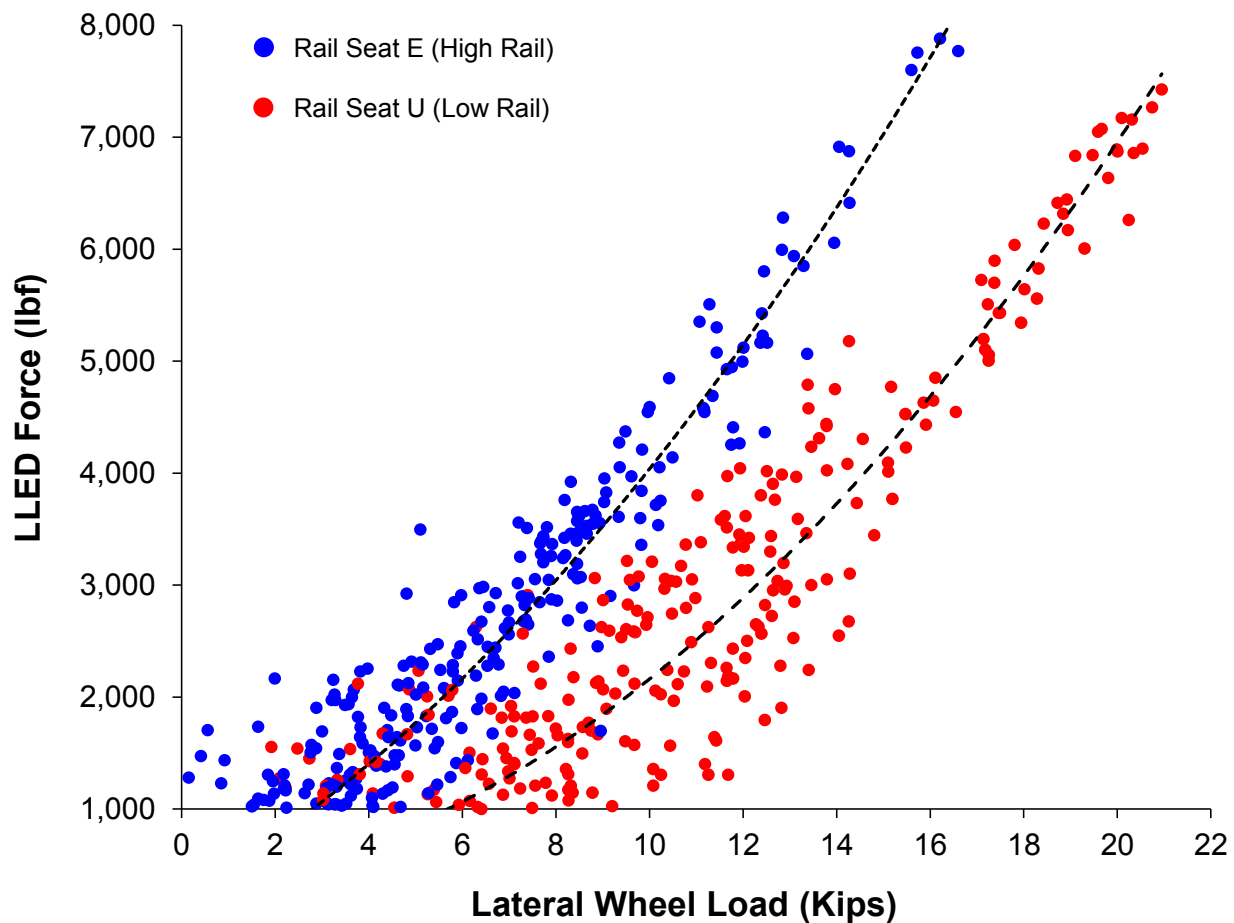
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15 **FIGURE 7 Change in lateral restraint forces as a function of lateral wheel load.**

16 **STATIC VERSUS DYNAMIC LATERAL RESTRAINT FORCES**

17 Figure 8 shows peak LLED forces under a HAL freight train from all axles and tested speeds as a
 18 function of lateral wheel load. The data indicate that at 20 kips of lateral wheel load, the lateral
 19 bearing restraint force will be approximately 7,000 lbf for rail seat U and 10,700 lbf when
 20 extrapolated for rail seat E. This would equate to a percentage of applied lateral wheel load

1 restrained by lateral bearing forces of 35% and 54% for rail seats U and E, respectively.
 2 Although the data cannot be directly compared due to different testing locations (RTT vs. HTL),
 3 it can be noted that rail seat U on the low rail of the HTL behaved similarly to the averaged data
 4 from the RTT. However, rail seat E on the high rail of the HTL produced much higher
 5 magnitudes of lateral bearing forces than the remaining data. Such a high percentage of lateral
 6 bearing forces under dynamic loads may be due to the rate of load application from the passing
 7 train and its effect on the frictional characteristics of the system, as well as the track geometry.
 8



9
 10 **FIGURE 8 Peak LLED forces under HAL freight train as a function of lateral wheel load.**

11 Although there appears to be a correlation between lateral bearing forces and lateral
 12 wheel loads, there is still a large amount of variability between the two metrics. Previous work
 13 has been conducted to understand the role friction plays on abrasion mechanisms of the concrete
 14 rail seat, as well as the role lateral fastening system stiffness plays on the magnitudes of lateral
 15 bearing forces (11, 13). Despite this work, a comprehensive understanding of the role friction
 16 plays in the transfer of lateral wheel loads into the fastening system does not exist (11, 13). This
 17 work will be continued with a stronger emphasis on the frictional characteristics of the fastening
 18 system and its effect on lateral restraint forces in concrete crosstie fastening systems.

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 21

1 CONCLUSIONS

2 Lateral Load Evaluation Devices (LLEDs) developed at UIUC have proven to be a successful
3 tool to quantify lateral restraint forces on the shoulder. Two governing equations, Equation 1: L_R
4 $= \Sigma L_B + \Sigma L_F$, and Equation 2: $L_F = \mu N$ guided this research to better understand how lateral
5 forces are restrained. Static and dynamic observations from the field show a high degree of
6 correlation and result in the following preliminary conclusions and observations:
7

- 8 • Under equivalent lateral loading conditions, lower magnitudes of vertical wheel load (i.e.
9 force applied normal to frictional planes) may result in higher lateral bearing restraint
10 forces due to the lower magnitude of force applied normal to the frictional planes.
11 However, the difference between constant 20 kip and 40 kip applied vertical wheel load
12 with increasing lateral wheel load is smaller than hypothesized.
13
- 14 • As lateral wheel load increases, the lateral frictional and bearing restraint forces begin to
15 converge. This indicates that the percentage of the applied lateral wheel load restrained
16 by frictional forces (ΣL_F from Equation 1) starts to decrease while the percentage of the
17 applied lateral wheel load restrained by bearing forces (ΣL_B from Equation 1) starts to
18 increase.
19
- 20 • As static lateral wheel load increases, the ratio of frictional forces to bearing forces
21 decreases from approximately 3.7 at 4 kips of lateral wheel load to 1.7 at 22 kips of
22 lateral wheel load, a decrease of 54%. The percentage of the applied lateral wheel load
23 restrained by lateral bearing restraint forces increases from approximately 21% at four
24 kips of lateral wheel load to 37% at 22 kips of lateral wheel load, an increase of 16%.
25
- 26 • In the dynamic lateral loading case, there is increased variability in the lateral loads
27 restrained by bearing forces resulting in increased demands on the fastening system
28 components. For example, the percentage of applied lateral wheel load restrained by
29 lateral bearing forces for rail seats U and E were 35% and 54%, respectively. Though rail
30 seat U exhibits behavior similar to the static results, the bearing force measured from rail
31 seat E exceeds the results from the static analysis by 17%.
32

33 In addition to further investigating the role of friction in the performance of the fastening
34 system through controlled laboratory experimentation, future work will include separating the
35 data by rolling stock type (i.e. locomotives, loaded freight cars, empty freight cars, and passenger
36 cars) to investigate the effects of varying train type and their mechanical characteristics. This
37 could provide railroads and suppliers with valuable insight into the demands that mixed traffic
38 impart on the track.

39 Future fastening system designs will benefit from the presented results by incorporating
40 the data into a mechanistic design approach by designing systems with optimum lateral load
41 transfer characteristics such as material selection and frictional properties. The results from
42 these LLED experiments will also be beneficial in validation of finite element modeling (FEM)
43 work also being conducted at UIUC. Moreover, the results will be used to aid in future fastening
44 system designs to mitigate current recurring problems seen on North American heavy-haul
45 freight railroads.
46

1 **ACKNOWLEDGEMENTS**

2 This project is sponsored by a grant from the Association of American Railroads (AAR)
3 Technology Outreach Program with additional funding for field experimentation provided by the
4 United States Department of Transportation (DOT) Federal Railroad Administration (FRA). The
5 published material in this paper represents the position of the authors and not necessarily that of
6 DOT. The authors would like to thank David Davis of AAR/TTCI and the members of the AAR
7 Technology Outreach Committee; John Bosshart and Thomas Brueske of BNSF; Harold
8 Harrison of H2 Visions, Inc.; Don Rhodes and Bill Rhodes of Instrumentation Services, Inc.;
9 Tim Prunkard, Darold Marrow, and Don Marrow of the University of Illinois at Urbana-
10 Champaign (UIUC); Jose Mediavilla of Amsted RPS; Jim Beyerl of CSX Transportation; Steve
11 Ashmore and Chris Rewczuk of UPRR; and Bob Coats of Pandrol USA for their advice,
12 guidance and contributions to this research. The authors would also like to acknowledge Daniel
13 Kuchma, David Lange, Thiago Bizarria, Christopher Rapp, Brandon Van Dyk, Sihang Wei,
14 Justin Grasse, Kartik Manda, and Andrew Scheppe from UIUC for their work on this research.
15 J. Riley Edwards has been supported in part by the grants to the UIUC Railroad Engineering
16 Program from CN, Hanson Professional Services, and the George Krambles Transportation
17 Scholarship Fund.

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